

GRAVITY TYPE WASTEWATER COLLECTION, TREATMENT, AND DISPOSAL SYSTEM IN K.HIMMAFUSHI

Detailed Design Report

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Project Title: Consultancy Services for Design and Works Supervision for Provision of Sewerage facilities in 05 (Five) Islands, Maldives.

Client: Ministry of Environment and Energy, Male', Republic of Maldives

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1 EXECUTIVE SUMMARY

This report addresses detailed design for the gravity type wastewater collection and disposal system for K.Himmafushi Island. The existing systems are mainly based small bore sewers. Without proper maintenance these systems are prone to damage and contamination of groundwater can occur. Therefore the current setup is not environmentally ideal. Furthermore, the consequential pollution of groundwater often renders the water unusable for cooking, drinking and other uses thereby adversely impacting the health and hygiene aspects of the community.

In order to establish a sustainable sewerage design, a concept design was initially prepared after compiling the data collected from a physical survey conducted during the early stages of the project. This survey aided in collecting information on existing sewerage system as well as clarifying the specific needs of the community. The detailed design is based on this approved concept presented. Furthermore, statistical projections on the population as well as households were calculated as primary information for the basis of the designing the sewerage network. Technical design parameters such as Average Dry Weather Flow (ADWF), Peak Dry Weather Flow (PDWF) and Peak Wet Weather Flow (PWWF) were compiled for this sewerage network.

The sewerage system for K.Himmafushi will be a gravity sewerage system where sloping pipelines will allow waste water from the entire island to flow to four zonal pumping stations. The pumping stations will transfer the sewage collected through a pressure main to the sewage treatment plant. The sewerage system shall comprise of elements of a given specification for sewer mains, cleanouts, manholes, vents, pump stations, outfall pipeline and diffuser that will ensure a 30 year design flow.

Based on design criteria and specifications, four zonal pumping stations are necessary to effectively service the entire island. Each pumping station will have 2 pumps each capable of pumping peak flow. A Sewage Treatment Plant (STP) of capacity 343 m³/d will be designed to treat the sewage before disposal through the sea outfall. In addition to this, an administration building will be located at the site to operate and maintain this network.

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2 INTRODUCTION

This is a detailed design report for the proposed Gravity Type Wastewater Collection and Disposal System for K.Himmafushi Island. This report will outline the system and infrastructure requirements necessary to implement a Gravity type sewer system. The detailed design is based on approved conceptual design adapted for this project and will be in compliance with the Design criteria and technical specifications for conventional gravity sewerage systems (EPA- Water and Sanitation unit).

2.1 BACKGROUND

2.1.1 Geographical Setting

The Island of Himmafushi, belonging to Kaafu Atoll, is located in the central region of Maldives at 4°18'35.79"N and 73°34'11.90"E. It is approximately 16 km north of the capital Male'. It has an area of about 33 hectares, occupied by a population of about 1070 as per the island council register. The island extends up to 1 km in length and a width of 0.89 km.

Table 1: Island Geographical Data

Island	Himmafushi
Atoll	Kaafu
Location	4°18'35.79"N , 73°34'11.90"E
Area	33 (hectares)
Length	1 km
Width	0.89 km

2.1.2 Existing sewerage facilities

The existing sewerage facilities on the island are based on small bore sewer systems. All the houses are connected via a small bore network which contains multiple outfalls. Poor maintenance and improper alignment in the gravity lines are identified has led to improper functioning of the system. This has resulted in damages to the pipeline contaminating the ground water. Consequently this may compromise the use of groundwater for cooking and other uses by the residents of the island. Therefore this is not environmentally ideal and unsafe for the health and hygiene aspects within the island.

2.1.3 Survey

During the topography survey road levels were taken at approximately 10 meter intervals. The survey reveals that the island has topography ranging from 0.5 m to 1.4 m from MSL. The island shows little variance in the elevations from zones and changes in elevation are gradual. Most areas of the island are flat with minor rise and fall in elevations. Taking these conditions into account, having two pump stations is economical and technically viable.

3 EXISTING & FUTURE POPULATION/HOUSING PROJECTIONS

3.1 Existing Population & Housing

According to the census result of 2014, total population of the island is 1308. However, the existing registered population is 1070 as per current register at Island Council. The total number of households including empty plots is 265. The population density of the island is 20 per hectare.

Table 2: Existing Population & Housing Figures (Island Council Registry)

Existing Population	1070
Population Density (per hectare)	32
Households	265

3.2 Future Population & Housing

According to statistics published by the National Planning Department, population growth rate of Himmafushi is 0.57%. However the island council register doesn't reflect the same growth rate. As such, the average growth rate of Maldives is used to estimate the future population. Based on this growth rate of 1.65%, the population of the island by the year 2031 and 2046 is expected to be 1368 and 1748 respectively.

Table 3: Future Projected Population

Year	2016	2031	2046
Growth Rate	1.65	1.65	1.65
Projected Population	1070	1368	1748

3.3 Updated Projected Connections

As per the current register at Island Council, total number of registered household is 265. However, projected connections for the 30 year design period are approximately 367 based on the average household size of 6.6.

4 WASTEWATER LOADING DESIGN CRITERIA

4.1 Average Dry Weather Flows (ADWF)

Average per capita wastewater generation will be taken as 120 lpcd for the design of *Average Dry Weather Flow (ADWF)* (Refer Table 5). This is a reasonable value suitable for islands with a small land area in the Maldives based on previous experiences.

For the purpose of estimating additional demand for industrial, commercial, institutional, parks and other uses will be taken from Table 4. This table refers to the Design criteria and technical specifications for conventional gravity sewerage systems guideline released by the Water and Sanitation Unit of the Environmental Protection Agency.

Table 4: Standard Wastewater Design Loadings (EPA Design criteria and technical specifications for conventional gravity sewerage systems)

Source/Development	Average Daily Flow L/unit	Unit
Auditorium/theater	10-15 L/day	Seat
Automobile repair garage	300 L/day	Garage
Carwash - garage	1000 L/day	Garage
Bakery	1000 L/day	Bakery
Cafeteria	100 L/day	Seat
Mosque	20 L/day	Person
Community centre	10-15 L/day	Person
Health Facility		Bed
Hospital	300 L/day	Bed
Laboratory	200 L/day	Laboratory
Manufacturing - industry	As per Assessment	
Office building	500 L/day	1000 square feet
Dormitory – college or residential	150 L/day	Student
Residential – boarding house	150 L/day	Bed
Residential – 1 bedroom apartment	150 L/day	Per person
Residential – 2-3 bedroom apartment	150 L/day	Per person
Residential – guest house	150 L/day	Per person
Restaurant – fixed seat	800 L/day	1000 square feet
School – day care center	20 L/day	Child
School – Kindergarten	20 L/day	Child
School – elementary/junior high	20 L/day	Student
School – high school	25 L/day	Student

For the design, utilization percentage considered in design will be 90% of the average daily demand (Refer Table 5).

Table 5: Average Dry Weather Flow (ADWF)

	15 Years	30 Years
Parameter	Design Value / Unit	Design Value / Unit
Average daily flow (ADF)	120 lpcd	120 lpcd
Design population	1368	1748
Utilization %	90%	90%
Institutional Demand	31 m ³ /day	34 m ³ /day
Demand for Correctional Services Buildings (Prison)	72 m ³ /day	90 m ³ /day
Average Dry flow (ADWF)	240 m ³ /day	301 m ³ /day

4.2 Peak Dry Weather Flow (PDWF)

Manning's Equation will be used to design the sewerage system. Peak Flow Factor shall be 4.0 as per Babbitt's formula for the 30 year project population at the growth rate of 1.65 %.

Table 6: Peak Dry Weather Flow (PDWF)

	15 Years	30 Years
Parameter	Design Value / Unit	Design Value / Unit
Daily average flow (ADWF)	240 m ³ /day	301 m ³ /day
Peak factor (PF)	4.0	4.0
Peak Dry flow (PDWF)	960 m ³ /day	1204 m ³ /day

4.3 Peak Wet Weather Flow (PWWF)

4.3.1 Inflow / Infiltration (Storm Allowances)

Sewer pipes have flexible joints, so for the purpose of estimating *Average Wet Weather Flows (AWWF)*, infiltration will be taken as 10% of ADWF and other flows such as storm water inflow will be 4% of ADWF (Refer Table 7).

Table 7: Peak Wet Weather Flow (PWWF)

	15 Years	30 Years
Parameter	Design Value / Unit	Design Value / Unit
Daily average flow (ADWF)	240 m ³ /day	301 m ³ /day
Infiltration	10%	10%
Other flows	4%	4%
Average Wet Flow (AWWF)	274 m ³ /day	343 m ³ /day
Peak factor (PF)	4.0	4.0
Peak Wet Flow (PWWF)	996 m ³ /day	1244 m ³ /day

The flow characteristic is primarily domestic and the average wet weather flow is 343 cubic meters per day and the corresponding peak flow rate is 14.4 litres per second for the 30 year projected population. However, all associated hardware including pumps and initial wastewater module will be designed for the first 15 year projected

population with average wet weather flow of 274 cubic meters per day and corresponding peak flow of 11.5 litres per second.

Table 8: Design Wastewater Loadings

	15 Years	30 Years
Parameter	Design Value / Unit	Design Value / Unit
Average Dry flow (ADWF)	240 m ³ /day	301 m ³ /day
Average Wet Flow (AWWF)	274 m ³ /day	343 m ³ /day
Peak Dry flow (PDWF)	960 m ³ /day	1204 m ³ /day
Peak Wet Flow (PWWF)	996 m ³ /day	1244 m ³ /day
Average flow	3.2 l/s	4.0 l/s
Peak flow	11.5 l/s	14.4 l/s

4.4 Water Design Allowances

In the design of the sewer system, allowances will be made for the leakage of groundwater into the sewers and building sewer connections (infiltration) and for other extraneous water entering the sewers from such sources as leakage through manhole covers, drains, roof down spouts, etc. Due to the extremely high peak flows that can result from roof down spouts, they should not, in any circumstances, be connected directly or indirectly via drains, to the sewer system.

5 SEWERAGE SYSTEM

5.1 System Overview

The sewerage system comprises of the following:

- Household inspection chambers,
- A gravity sewerage reticulation network,
- Reticulation pumping stations and pressure main system.

The sewerage system for K.Himmafushi will be a gravity sewerage system. Wastewater from the entire island will be drained using a gravity pipe system to zonal pumping stations. The Pumping mains will transfer the sewage to the Wastewater treatment plant where by the sewage will be treated and the remaining effluent will be discharged into the sea directly through sea outfall. Pump stations located at each catchment will be able to effectively cater the wastewater generated at Average Wet Weather Flows.

The sewerage system design will be based on an assumption velocity of at least 0.60m/sec, with a pipe slope of 1 in 250 throughout, and access chamber spacing of not more than 60m, and an inspection chamber within the boundary perimeter of each house.

5.2 Sewerage System Components

5.2.1 Sewer Mains

The length of the gravity sewer main required is approximately 14352 km. Minimum uPVC pipe diameter for house laterals will be 110mm OD. Minimum main sewer uPVC pipe diameter used will be 160mm OD and above as per hydraulic design. Force main pipes will be of HDPE material and discharge capacity of sewer depends on size of sewer and flow velocity. Pipe cover over sewer pipes are of 600mm, unless deemed necessary onsite in a special case. Minimum slope required for 160 mm diameter pipe conventional sewer of 1 in 250 (0.4%) is maintained to lay the pipes within a depth range of 0.6m to 2.7m. However, the slope is adjusted if the minimum cover could not be achieved. As such few of the main lines will be at slopes greater than 0.4%. Due to the small gradient and limited sewage generated at the current flow levels, minimum velocity could not be maintained at upper reaches. Average present flow is 2.4 l/s and ultimate flow is 8.8 l/s.

5.2.2 Cleanouts, and Manholes

5.2.2.1 Cleanouts (CO)

Cleanouts or rodding points will be installed at the start of all main sewer lines with a 160mm OD PVC 45 degree bend, a threaded end cap and a rubber gasket.

5.2.2.2 Manholes

Pre - fabricated HDPE circular shafts for manholes and inspection chambers of minimum clear opening access 600mm diameter as standard size will be used and will be of non-biodegradable and resistant to salt. The manholes are at a maximum of 60m intervals and at every road junction. The manholes/access chamber covers is designed for heavy duty load.

5.2.2.3 Bends

All bends will be provided with reaction blocking, tie rods or restrained joints designed to prevent movement.

5.2.3 Vents

Vents in household plumbing are sufficient. However to avoid foul gases inside the network and pump stations, a 9m high GI pipe vent stack will be provided at the pumping stations.

5.2.4 Pumping Stations (or Lifting Stations)

The pumping station will be comprised of a wet well, submersible sewage pumps (one duty and one standby) and an adjacent valve chamber/discharge chamber. The pumping station will be below ground level with an adjacent weatherproof and vandal proof cabinet housing the pump controls. Locations of the pumping stations were decided after a meeting with the Island Council and approved pumping station. Locations of the pump stations are provided in Annex III.

Four zonal stations are required to effectively service the entire network based on the design criteria and specifications. Each pumping station will have 2 pumps (one duty and one standby) each capable of pumping peak flow. Since the pump life is approximately 5 years, pumps will be selected for the 15 year design flow.

5.3 Other Considerations

5.3.1 Emergency Storage

Pumping stations will be designed with emergency storage capacity to retain the maximum dry weather sewage inflow for a minimum response time of two hours.

5.3.2 Emergency Overflow System

5.3.2.1 Catchment Overflow Links (LO)

All catchments will be interlinked via a catchment overflow 160mm OD pipe laid at a flat grade at the closest point between two catchments to provide relief during an event where the pump station in a given catchment fails.

5.3.2.2 Emergency Flow Relief Structures (EFRS)

Emergency flow relief structures will not be provided since, the pumping stations will be designed for the emergency storage to retain two hours peak dry weather flow to

provide an adequate response time to a pump station failure. In addition, pumping stations will be provided with an Emergency bypass mechanism for use during pump station service and maintenance.

5.4 Connections to the Sewer System

5.4.1 Residential Connections

All premises will be connected to the sewer main with uPVC pipe of 110mm OD gravity sewer laid at a minimum grade of 1.5-2.5% to facilitate the flow of solids. A pre-fabricated corrugated uPVC inspection chamber of 315mm OD will be placed at the boundary of each residential connection prior to downstream connection with a 'Y' fixture, lateral connection.

5.5 Preliminary hydraulic design of Sewers Network

5.5.1 Design Criteria

The following parameters are taken into consideration for the design of the sewers network.

Table 9: Design Criteria

	15 Years	30 Years
Parameter	Design Value / Unit	Design Value / Unit
Design Population	1368	1748
Average daily flow (ADF)	120 lpcd	120 lpcd
Utilization %	90 %	90 %
Infiltration	10 %	10 %
Other flows	4 %	4 %
Peak factor (PF)	4.0	4.0
Pipe size	160mm OD	160mm OD
Pipe material	uPVC	uPVC
Pipe slope	1:250	1:250
Min. depth of pipe	0.6m	0.6m
Max. depth of pipe	2.7m	2.7m
Max. depth of excavation	3.0 - 3.5m	3.0 - 3.5m
Length of sewer mains	14.3 km	14.3 km
Peak Wet Flow (PWWF)	727 m ³ /day	909 m ³ /day

5.5.2 Concept hydraulic design of various components of sewer network

The average flow taken for the hydraulic design of the system is 120 litres per capita per day. In addition to this institutional demands are calculated based on the EPA guidelines. The estimate of 120 LPCD is a reasonable value appropriate for small islands of Maldives and 90 percent utilization is considered for the design. The entire island will be divided into two catchments in order to effectively service the network based on the design criteria and specifications. Each pumping station will have a pump, each capable of pumping peak flow and pumps will be selected for the 15 year design flow. The minimum

diameter of the pump well will be 2.5m and depth limited to a maximum of 3.6m as recommended in the guideline.

6 GENERAL DESIGN CONSIDERATIONS FOR PUMP STATIONS AND RISING MAINS

6.1 Reticulation Pumping Stations & Valve Chambers

Four zonal pumping stations are required to effectively service the entire network based on the design criteria and specifications. Four zonal pumps are required because the land reclamation area and the jail/rehabilitation area will also be part of the system. It is required to install pump stations at these positions as well for optimum performance.. The pumping mains will be design in such a way that future expansion to include the reclamation area and rehabilitation area can be accommodated easily. An external valve chamber will service each of the pump stations. Locations of the pumping stations were decided after a meeting with the Island Council.

6.1.1 Wet Well Design Requirements

The minimum diameter of the wet well will be 2.0m and depth limited to a maximum of 3.5m. The pump well floor is to be shaped to avoid retention of waste water and material entering the pump well. Where this is achieved through placement of mass concrete, the concrete is to be sloped towards the well pump sump. The design will allow pumps and personnel can pass through the pump well opening.

6.1.1.1 Control Levels

Normal pump operation will be controlled through float-switches. The pump control system will consist of the following components for float switches:

- Low level to signal pump OFF,
- High level to signal duty pump ON,
- Maximum level alarm and,
- Standby pump ON.

The pump well will be capable of accommodating two submersible pumps, associated pipe work, electrical wiring and access equipment and personnel access as shown in Annex I.

6.1.2 Construction requirements

Pump stations will be constructed using reinforced concrete and internal surface will be provided with a double mat GRP lining with gel coat in accordance with the manufacturer's instructions.

6.2 Rising Mains

Rising Mains or Pressure Mains used for the network will be of HDPE/PE and a diameter of 110mm OD pipe with a specified wall thickness and stiffness to satisfy PE100 PN10 classification for resistance to abrasion and ease of jointing using fusion butt welding.

6.2.1 Discharge Pipework Velocities

Velocities in the rising mains shall be maintained at a minimum value of 2.0m/s to facilitate the removal of sedimentation from valves and a maximum value of 4.0m/s to

avoid scouring of pipe walls. Thrust blocks will be provided for the discharge pipes at not less than 1.0m intervals in any direction.

6.2.2 Isolation & Control Valves

An isolation and control valves will be placed at upstream of the rising main intersection inside the valve chamber and at downstream end of pump well in order to isolate the connection in case of pipe failure or maintenance.

6.1 Pump Selection for pump stations

The initial sizing of pumps is based on the 15 year demand. However the pump sizing for the 30 year are also provided in the table below. Details of the pump selection is provided in the Annex II. Pump curves are also provided. It should be noted that two pumps will be installed in one station.

Table 10 -Pump Selection

Pump station	15 Year Period		30 Year Period	
	Design Flow (l/s)	Design Head (m)	Design Flow (l/s)	Design Head (m)
PS1	4.1	26.5	9.0	5.1
PS2	3.0	20.9	6.2	3.8
PS3	2.2	24.9	4.0	2.8
PS4	3.4	16.9	4.4	4.3

6.1.1 Control Levels

Normal pump operation will be controlled through float-switches. The pump control system will consist of the following components for float switches:

- Low level to signal pump OFF,
- High level to signal duty pump ON,
- Maximum level alarm and,
- Standby pump ON.

The pumps specifications will be as follows:

- Pump type: Submersible grinder capable of handling solids up to 75mm
- Impeller: SS ASTM A743 CF 8 M
- Shaft: SS BS 970 Gr 304 S11
- Casing: IS 210 Gr.FG 260 with 1.5-2% Ni
- Start/Stops: restricted to 6 per hour
- Motor: [as per engineering design], IP 68 protection with Class F insulation
- Guiderail pipe & Chain: SS BS 970 Gr 304 S11

All equipment such as the inlet chamber, pump wells, valves, pumps, control cabinets/panels, control cables, instrument protection, indicator meters, switches, lights, control system, ultra sensors/switches, float switches, pump controllers and related accessories will be in conformance with the published guidelines in Maldives.

7 SEWAGE TREATMENT FACILITY

Sewerage Treatment Plant will be installed in this project. For the discharged water quality, the limits defined by the European Normative 91/271/EEC and by the Water and Sanitation Unit of Maldives EPA guideline, Design criteria and technical specifications for conventional gravity sewerage systems, are as follows:

Table 11: Influent and Effluent Standards

<i>Influent Data</i>		<i>Effluent Data</i>	
<i>pH</i>	6.8 – 8.5		
<i>BOD₅</i>	300 mg/l	<i>BOD₅</i>	20 mg/l
<i>COD</i>	500 mg/l	<i>COD</i>	125 mg/l
<i>TSS</i>	250 mg/l	<i>TSS</i>	20 mg/l
<i>TKN</i>	50 mg/l	<i>TKN</i>	15 mg/l
<i>Total phosphorus</i>	7 mg/l	<i>Total phosphorus</i>	2 mg/l
<i>Oils and fats</i>	15 – 30 mg/l	<i>Oils and fats</i>	2 mg/l

7.1 Activated Sludge Treatment Plant

Preliminary capacity of Extended Aeration system will be 343 m³/day, and the detailed layout is presented in annex I. The overall process associated with the treatment plant are as follows.



Figure 1: Activated Sludge process diagram

7.1.1 Bar screen and the oil and grease trap.

Wastewater will be pumped from pump stations in to the oil separator tank after passing through coarse bar screen. The bar screen will have a manually cleanable screen, a walk way to facilitate the operator to reach the bar screen, remove and replace the screens when necessary, and the system shall be divided into two equal units which can be isolated separately for maintenance. The oil and grease will be separated from the OG trap by manual scooping of the free floating oil and grease layer. The grease trap will house a submersible pump to transport the sludge accumulated at the bottom to the equalization tank.

Table 12-Screening Parameters

Raking Mechanism	Manual
Number of Screens	2 (1 per unit)
Screen Perforations	6-10 mm
Encasement	Stainless Steel

7.1.2 Equalization Tank (EQT)

Once the oil and grease has been screened the wastewater will flow into equalization tank. The equalization tank will have the capacity to hold the flow of 15mins of peak flow which is equal to about an hour of AWWF. Each compartment of the EQT will have a submersible pump which will pump water into the aeration tank. Flow meters will be placed between equalization tank and the aeration tank and actuator valve will help to control the flow from EQT to aeration tank.

7.1.3 Aeration tank

From EQT wastewater will be pumped to the Aeration tank through a Flow Controller mechanism which can be monitored from the control room. The aeration tanks will have a retention time of 18- 20 hours.

The aeration tanks will be equipped with fine bubble generation system. Ceramic fine bubble diffusers will be fitted to base of the aeration tank. Air will be supplied with the help of air blowers which will be housed in the administration building. Air blowers will be twin lobe design coupled with motor, pulleys, filter, silencer and baseplate as Design Criteria and Technical Specification for Conventional Gravity Systems (EPA, MEE, and Water and sanitation unit). Blowers with sufficient power will be used to provide the necessary aeration which will help in bacterial growth. The purpose of aeration is to provide the desired effects for biological treatment. The process is based on extended aeration enable the volume of sludge produced to be considerably restricted and efficiency of biological treatment caters to large buffer capacity and cope with peak flows encountered. The foul odor or smell will be kept at the minimum possible as the system has large amount of bacteria growth and oxygen presence in the system.

7.1.4 Sedimentation tank (clarifier tank)

The aerated effluent from the aerator is let into clarifier unit by gravity. The sludge at a bottom level is sent back in to the sludge sump and finally into the drying beds for disposal. The system is designed such a way that any excess sludge can be diverted in to aeration tank. Sedimentation tank will have a minimum of 4 hour retention.

7.1.5 Sludge holding tank

The sludge collected at the bottom of the clarifier is passed to sludge holding tanks. Sludge holding tanks are constructed below ground level. The sludge holding tanks have pumps to recirculate the sludge into aeration tank to attain the sludge retention time (SRT)

required. Once the SRT is attained the waste activated sludge will be transported to sludge drying beds.

7.1.6 Clear water tank

The clear water that overflows into effluent launder will flow by gravity into the clear water tank. The clear water tank holds the effluent before being discharged via sea outfall.

7.1.7 Sludge drying beds

The waste activated sludge is dried using drying beds and the sludge cake produced will be sent to solid waste management facility. The effluent from the sludge drying beds will be collected to an effluent tank, which will have pumps to recirculate the effluent back into aeration tank.

7.1.8 Sea outfall pump.

Two sea outfall pumps will provide the necessary pressure to discharge the effluent into sea. These will be housed in the administration building.

	15 Year Period		30 Year Period	
	Design Flow (l/s)	Design Head (m)	Design Flow (l/s)	Design Head (m)
16.3	12.7	24.1	15.9	24.1

7.2 Outfall Pipeline and Diffuser

7.2.1 Outfall Pipeline

The length of the sea outfall is approximately 850 m. The diameter of the black PE100 pipes used will be 160mm OD. Outfall pipe will be laid to depth of 0.6 m on land and place on the natural sea bed using concrete ballast blocks anchored to the seabed to prevent the movement of the pipeline during heavy wave activity. The pipe joints will be HDPE fusion welded.

7.2.2 T-Head Diffuser Arrangement

Outfall "T" diffuser will be fixed at end of pipe outside of the reef which is at a minimum depth of 6m below lowest tide level. However, it is expected that the end of the outfall pipe to be at 15m depth below the lowest tide level to ensure maximum environmental benefit.

7.2.3 Shoreline Stability

For the purpose of minimum impact on shoreline, the depth of pipe portion laid up to beach toe will be placed on same level as the seabed level. Pipe installed on the sea portion will be using concrete ballast blocks anchored to the seabed.

8 ADMINISTRATION BUILDING

8.1 Administration Building and Standby Arrangements

An administration building will be located at the site to facilitate the maintenance and operational works. The administration building will also serve as a storage facility with sufficient covered vehicle parking space for a standby generator set and service vehicle.

8.2 Power Supply & Upgrade Requirements

8.2.1 Existing Power Infrastructure

K.Himmafushi has electricity for 24 hours generated from the powerhouse. The powerhouse has four generators to power the island with a total Power Generation capacity of 1010KW. No upgrades to the system are envisaged to cater for the sewerage system as current power supply system has the capacity to provide for the pump stations.

8.2.2 Power Supply Upgrades & Connection Requirements

Estimated power consumption for the sewerage system will be 80 kW.

8.2.3 General Electrical Arrangement

The pumping stations are supplied with electricity from the existing island power supplies. Mobile backup generator will be included as a backup during emergencies.

9 OPERATION AND MAINTENANCE

9.1 Operation and Maintenance Requirements

9.1.1 Sewer Cleaning

Difficulties arising due to low velocities observed and its resulting tendency for deposition are tackled in two ways; the high density of population and the close proximity of incoming house laterals leads in flushing pipe system. Annual hydro jetting of the network is recommended for further cleansing of the network.

9.1.2 Sewer System Maintenance Tools

All necessary maintenance tools such as squeegees, wrenches, valve keys, rakes, shovels, spare parts, etc. will be provided. A sewer jetting machine is desirable. Readily accessible storage space and workbench facilities will be provided and consideration will be given to provision of a garage for large equipment storage, maintenance and repair.

Maintenance tools and spare parts for the wastewater system will be provided by the Contractor for the operation period of one year.

Table 13 - Sewer System Maintenance Tools & Spare Parts

Maintenance Tools	Qty
Trailer mounted sewer jetting machine	1 nos
Adjustable wrench made up of carbon steel with standard pattern of size 200mm	2 nos
Double ended open jaw spanner set of sizes (mm): 6x7, 8x9, 10x11, 12x13, 14x15, 16x17, 18x19, 20x21, 22x23, 24x25, 26x27, 28x29, 30x31, 32x33	1 nos
Screw driver set consisting of various sizes (6", 8", 12")	1 nos
Cutting Pliers 12"	1 nos
Hacksaw frame with blade	1 nos
Mechanical components	
Spare parts of each pump	2 sets
Float Switch	2 nos
Electrical components	
Contactor MN16	1 nos
Contactor MN12	1 nos
230 -V-AC Coil for Contactors	2 nos
Overload Relay MN2	2 nos
1-0-2 Change Over switch 16 Amps	2 nos
Panel Board indicator	5 nos
Single phase preventer	1 nos

Table 14-Safety tools

Safety Tools	Qty
5 kg capacity dry chemical powder type Fire Extinguisher	2 nos
Wooden First Aid Box	2 nos
Fire Bucket sand with 3 nos. of round bottom buckets	2 nos
Rubber mat of 1.1kV grade of size 6mm thick	2 nos
Fire Safety Rules chart	2 nos
Shock Treatment Chart	2 nos
Manuals and Catalogues	2 nos

9.2 Training of Personnel

As per technical specification, training will be provided by the Contractor as follows:

1. A 3 months comprehensive training program facilitated by a skilled operator will be provided for local operations and maintenance personnel following the signing of handing over Certificates. The training will include practical training on all aspects of the operation, maintenance and routine repair of the whole sewerage collection and pumping network, sewage pumping stations, equipment and facilities under normal and special operating conditions. The training will include but is not limited to training related to process, mechanical, electro-mechanical, electrical, instrumentation and control equipment supplied and installed.
2. For the purpose of training to maintain the said facilities and equipment, proper training manuals based on operation and maintenance manual and checklists will be provided.
3. An awareness and training program for the Island community, Island Office staff, utility company staffs and other personnel that may be in-directly involved with the maintenance of the household sanitation and public sewers will be provided. The awareness program would include aspects related to community participation during the construction since most of the work will be executed within or close to the houses and properties to be connected.

10 ANNEX I – DETAILED DRAWINGS

11 ANNEX II – PUMPING MAIN CALCULATIONS

PUMPING STATION FLOW

	q pk	Inlet Depth	Legth riser
	(l/s)	m	m
PS1	5.1	2.6	952
PS2	3.8	2.7	568
PS3	2.8	2.4	837
PS4	4.3	2.7	305
SO	15.9	1.0	850

SEWERAGE PUMPING STATION DESIGN

PS	Length p.main	Diamete r p.main	Inlet depth	Diamete r well	Area	q	Depth req.	v	Design depth
	m	m	m	m	m2	l/s	m	m/s	m
PS1	952	0.13	2.6	2.5	4.9	10.1	3.7	1.6	3.6
PS2	568	0.13	2.7	2.5	4.9	7.5	3.6	1.2	3.6
PS3	837	0.13	2.4	2.5	4.9	5.5	3.3	0.9	3.6
PS4	305	0.13	2.7	2.5	4.9	8.6	3.7	1.3	3.6

SEWERAGE PUMPING STATION PUMP SELECTION (30 YEARS)

	Factor	Diamete r p. main	Length p.main	Q	Q	H
	k	mm	m	m3/min	l/s	m
PS1	0.163	160	952	0.6	10.1	26.5
PS2	0.163	160	568	0.5	7.5	20.9
PS3	0.163	160	837	0.3	5.5	24.9
PS4	0.163	160	305	0.5	8.6	16.9
SO	0.163	200	850	1.0	15.9	24.1

CUMULATIVE HEAD LOSS (30 YEARS)

PS1				
Peak Flow, q (l/s)	10.14			
Pipe Diameter ID (mm)	90			
Description	Unit	No.	Factor	HL
Static head	m	3.5	1	11.2
90 deg Elb.	nos.	6	0.05	0.15
Reflux Valve	nos.	2	0.2	0.4
T' Junction	nos.	1	0.1	0.1
Pipe Run to 'T'	m	6	0.004	0.02
Pipe Run After 'T'	m	952	0.015	14.28
90 deg Elb.	nos.	6	0.05	0.3
TOTAL				26.45

PS2				
Peak Flow, q (l/s)	7.52			
Pipe Diameter ID (mm)	90			
Description	Unit	No.	Factor	HL
Static head	m	3.5	1	11.2
90 deg Elb.	nos.	6	0.05	0.15
Reflux Valve	nos.	2	0.2	0.4
T' Junction	nos.	1	0.1	0.1
Desity factor	m	6	0.03	0.18
Pipe Run After 'T'	m	568	0.015	8.52
90 deg Elb.	nos.	6	0.05	0.3
TOTAL				20.85

PS3				
Peak Flow, q (l/s)	5.50			
Pipe Diameter ID (mm)	90			
Description	Unit	No.	Factor	HL
Static head	m	3.5	1	11.2
90 deg Elb.	nos.	6	0.05	0.15
Reflux Valve	nos.	2	0.2	0.4
T' Junction	nos.	1	0.1	0.1
Desity factor	m	6	0.03	0.18
Pipe Run After 'T'	m	837	0.015	12.56
90 deg Elb.	nos.	6	0.05	0.3
TOTAL				24.89

PS4				
Peak Flow, q (l/s)	8.58			
Pipe Diameter ID (mm)	90			
Description	Unit	No.	Factor	HL
Static head	m	3.5	1	11.2
90 deg Elb.	nos.	6	0.05	0.15
Reflux Valve	nos.	2	0.2	0.4
T' Junction	nos.	1	0.1	0.1
Desity factor	m	6	0.03	0.18
Pipe Run After 'T'	m	305	0.015	4.58
90 deg Elb.	nos.	6	0.05	0.3
TOTAL				16.91

PUMPING STATION FLOW

	q pk	Inlet Depth	Legth riser
	(l/s)	m	m
PS1	4.1	2.6	952
PS2	3.0	2.7	568
PS3	2.2	2.4	837
PS4	3.4	2.7	305
SO	12.7	1.0	850

SEWERAGE PUMPING STATION DESIGN

PS	Length p.main	Diamete r p.main	Inlet depth	Diamete r well	Area	q	Depth req.	v	Design depth
	m	m	m	m	m2	l/s	m	m/s	m
PS1	952	0.13	2.6	2.5	4.9	8.1	3.6	1.3	3.6
PS2	568	0.13	2.7	2.5	4.9	6.0	3.6	0.9	3.6
PS3	837	0.13	2.4	2.5	4.9	4.4	3.2	0.7	3.6
PS4	305	0.13	2.7	2.5	4.9	6.9	3.6	1.1	3.6

SEWERAGE PUMPING STATION PUMP SELECTION (15 YEARS)

	Factor	Diamete r p. main	Length p.main	Q	Q	H
	k	mm	m	m3/min	l/s	m
PS1	0.163	160	952	0.5	8.1	26.5
PS2	0.163	160	568	0.4	6.0	20.9
PS3	0.163	160	837	0.3	4.4	24.9
PS4	0.163	160	305	0.4	6.9	16.9
SO	0.163	200	850	0.8	12.7	24.1

CUMULATIVE HEAD LOSS

(15 YEARS)

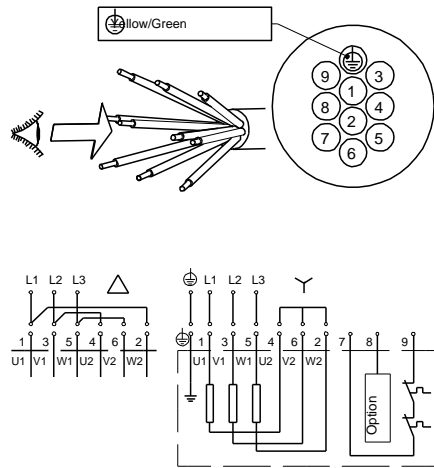
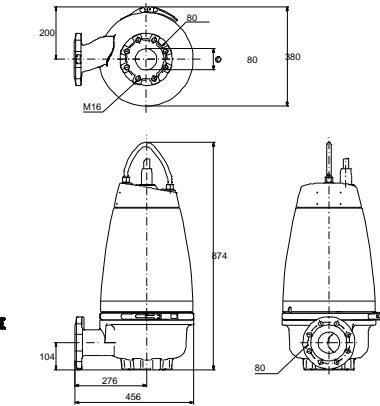
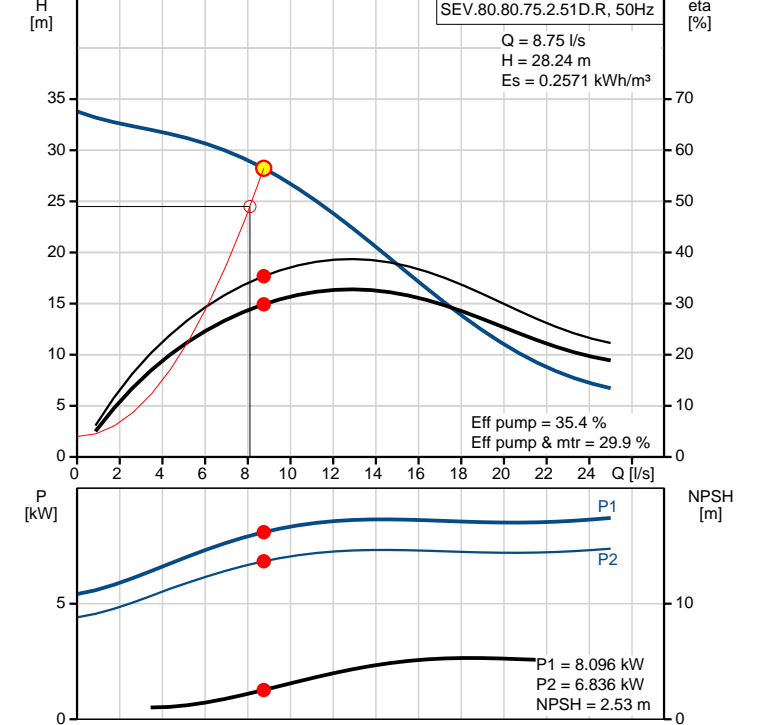
PS1				
Peak Flow, q (l/s)	8.10			
Pipe Diameter ID (mm)	90			
Description	Unit	No.	Factor	HL
Static head	m	3.5	1	11.2
90 deg Elb.	nos.	6	0.05	0.15
Reflux Valve	nos.	2	0.2	0.4
T' Junction	nos.	1	0.1	0.1
Pipe Run to 'T'	m	6	0.004	0.02
Pipe Run After 'T'	m	952	0.015	14.28
90 deg Elb.	nos.	6	0.05	0.3
TOTAL				26.45

PS2				
Peak Flow, q (l/s)	6.02			
Pipe Diameter ID (mm)	90			
Description	Unit	No.	Factor	HL
Static head	m	3.5	1	11.2
90 deg Elb.	nos.	6	0.05	0.15
Reflux Valve	nos.	2	0.2	0.4
T' Junction	nos.	1	0.1	0.1
Desity factor	m	6	0.03	0.18
Pipe Run After 'T'	m	568	0.015	8.52
90 deg Elb.	nos.	6	0.05	0.3
TOTAL				20.85

PS3				
Peak Flow, q (l/s)	4.40			
Pipe Diameter ID (mm)	90			
Description	Unit	No.	Factor	HL
Static head	m	3.5	1	11.2
90 deg Elb.	nos.	6	0.05	0.15
Reflux Valve	nos.	2	0.2	0.4
T' Junction	nos.	1	0.1	0.1
Desity factor	m	6	0.03	0.18
Pipe Run After 'T'	m	837	0.015	12.56
90 deg Elb.	nos.	6	0.05	0.3
TOTAL				24.89

PS4				
Peak Flow, q (l/s)	6.86			
Pipe Diameter ID (mm)	90			
Description	Unit	No.	Factor	HL
Static head	m	3.5	1	11.2
90 deg Elb.	nos.	6	0.05	0.15
Reflux Valve	nos.	2	0.2	0.4
T' Junction	nos.	1	0.1	0.1
Desity factor	m	6	0.03	0.18
Pipe Run After 'T'	m	305	0.015	4.58
90 deg Elb.	nos.	6	0.05	0.3
TOTAL				16.91

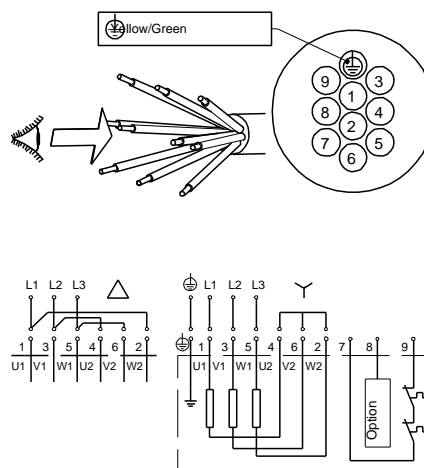
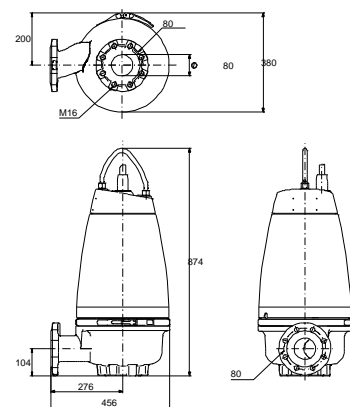
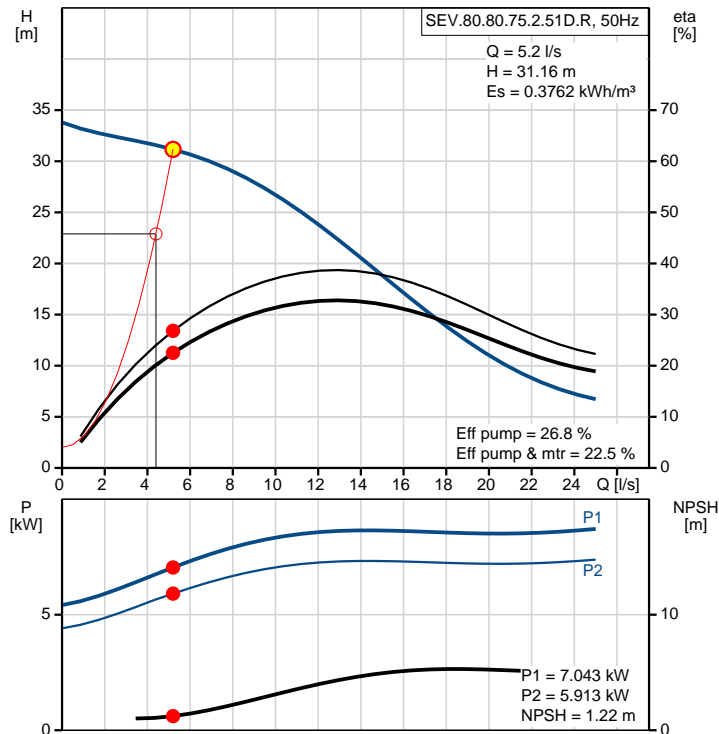
Description	Value
General information:	
Product name:	SEV.80.80.75.2.51D.R
Product No.:	96889330
EAN:	5700312864462
Price:	On request
Technical:	
Actual calculated flow:	8.75 l/s
Max flow:	25 l/s
Resulting head of the pump:	28.24 m
Head max:	33.8 m
Type of impeller:	Super Vortex
Maximum particle size:	80 mm
Primary shaft seal:	SIC/SIC
Secondary shaft seal:	CARBON/CERAMICS
Max. hydraulic efficiency:	38 %
Approvals on nameplate:	EN12050-1
Curve tolerance:	ISO9906:2012 3B2
Cooling jacket:	with cooling jacket
Materials:	
Pump housing:	EN 1.4408
Impeller:	Stainless steel
Installation:	
Maximum ambient temperature:	313 K
Maximum operating pressure:	600 kPa
Flange standard:	DIN
Pump outlet:	DN 80
Pressure stage:	PN 10
Maximum installation depth:	20 m
Inst dry/wet:	DRY/SUBMERGED
Installation:	horizontal or vertical
Liquid:	
Pumped liquid:	any viscous fluid
Liquid temperature range:	273 .. 313 K
Density:	998.2 kg/m³
Electrical data:	
Power input - P1:	8.9 kW
Rated power - P2:	7.5 kW
Main frequency:	50 Hz
Rated voltage:	3 x 380-415 V
Voltage tolerance:	+6/-10 %
Max starts per. hour:	20
Rated current:	16,5-16,2 A
Rated current at 3/4 load:	12.7 A
Rated current at 1/2 load:	10.4 A
Starting current:	152 A
Rated current at no load:	7.8 A
Cos phi - power factor:	0,83
Cos phi - p.f. at no load:	0,14
Cos phi - p.f. at 3/4 load:	0,76
Cos phi - p.f. at 1/2 load:	0,65
Rated speed:	2940 rpm
Locked-rotor torque:	80 Nm
Breakdown torque:	112 Nm
Moment of inertia:	0.0215 kg m²
Motor efficiency at full load:	84.8 %
Motor efficiency at 3/4 load:	83.8 %
Motor efficiency at 1/2 load:	80.1 %
Number of poles:	2
Start. method:	star/delta
Enclosure class (IEC 34-5):	IP68
Insulation class (IEC 85):	F
Explosion proof:	no
Motor protection:	THERMAL SWITCH
Thermal protec:	internal
Length of cable:	10 m
Cable type:	LYNIFLEX
Type of cable plug:	NO PLUG



Description	Value
Controls:	
Control box:	not included
Moisture sensor:	without moisture sensors
Water-in-oil sensor:	without water-in-oil sensor
Temp. sensor:	N
Others:	
Net weight:	142 kg

Description	Value
Controls:	
Control box:	not included
Moisture sensor:	without moisture sensors
Water-in-oil sensor:	without water-in-oil sensor
Temp. sensor:	N
Others:	
Net weight:	142 kg

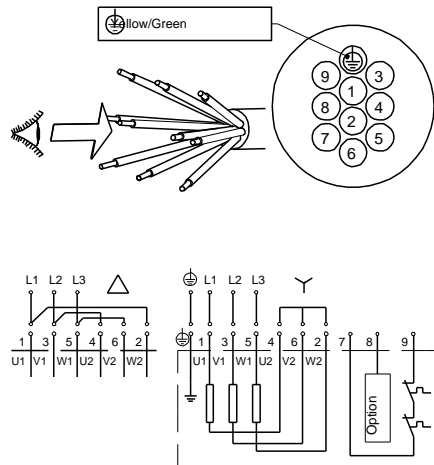
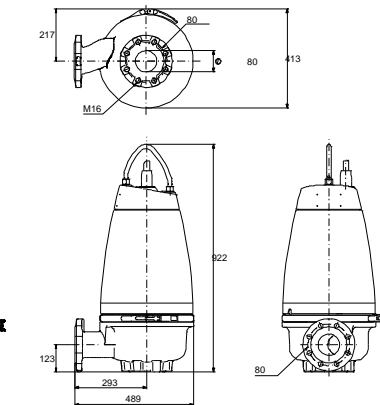
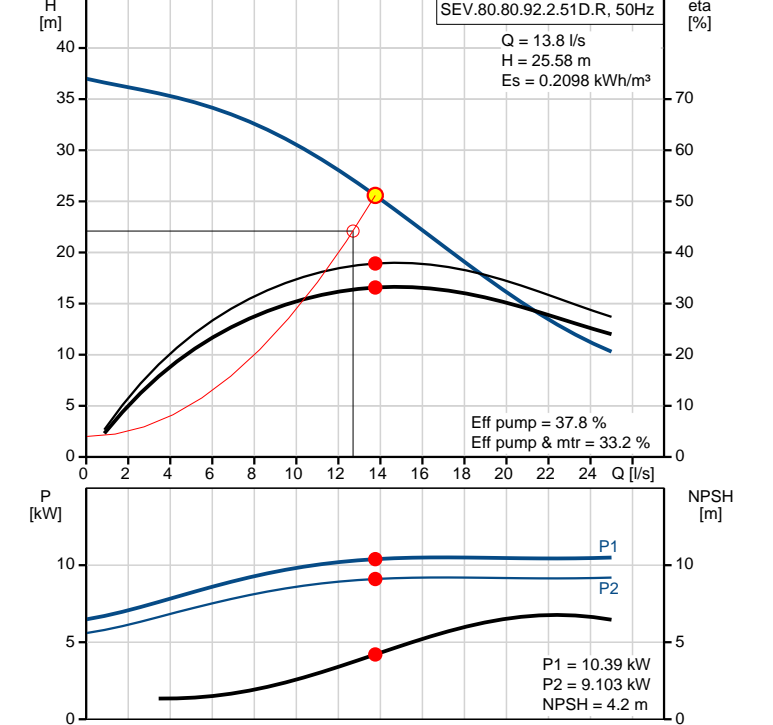
Description	Value
General information:	
Product name:	SEV.80.80.75.2.51D.R
Product No.:	96889330
EAN:	5700312864462
Price:	On request
Technical:	
Actual calculated flow:	5.2 l/s
Max flow:	25 l/s
Resulting head of the pump:	31.16 m
Head max:	33.8 m
Type of impeller:	Super Vortex
Maximum particle size:	80 mm
Primary shaft seal:	SIC/SIC
Secondary shaft seal:	CARBON/CERAMICS
Max. hydraulic efficiency:	38 %
Approvals on nameplate:	EN12050-1
Curve tolerance:	ISO9906:2012 3B2
Cooling jacket:	with cooling jacket
Materials:	
Pump housing:	EN 1.4408
Impeller:	Stainless steel
Installation:	
Maximum ambient temperature:	313 K
Maximum operating pressure:	600 kPa
Flange standard:	DIN
Pump outlet:	DN 80
Pressure stage:	PN 10
Maximum installation depth:	20 m
Inst dry/wet:	DRY/SUBMERGED
Installation:	horizontal or vertical
Liquid:	
Pumped liquid:	any viscous fluid
Liquid temperature range:	273 .. 313 K
Density:	998.2 kg/m³
Electrical data:	
Power input - P1:	8.9 kW
Rated power - P2:	7.5 kW
Main frequency:	50 Hz
Rated voltage:	3 x 380-415 V
Voltage tolerance:	+6/-10 %
Max starts per. hour:	20
Rated current:	16,5-16,2 A
Rated current at 3/4 load:	12.7 A
Rated current at 1/2 load:	10.4 A
Starting current:	152 A
Rated current at no load:	7.8 A
Cos phi - power factor:	0,83
Cos phi - p.f. at no load:	0,14
Cos phi - p.f. at 3/4 load:	0,76
Cos phi - p.f. at 1/2 load:	0,65
Rated speed:	2940 rpm
Locked-rotor torque:	80 Nm
Breakdown torque:	112 Nm
Moment of inertia:	0.0215 kg m²
Motor efficiency at full load:	84.8 %
Motor efficiency at 3/4 load:	83.8 %
Motor efficiency at 1/2 load:	80.1 %
Number of poles:	2
Start. method:	star/delta
Enclosure class (IEC 34-5):	IP68
Insulation class (IEC 85):	F
Explosion proof:	no
Motor protection:	THERMAL SWITCH
Thermal protec:	internal
Length of cable:	10 m
Cable type:	LYNIFLEX
Type of cable plug:	NO PLUG



Description	Value
Controls:	
Control box:	not included
Moisture sensor:	without moisture sensors
Water-in-oil sensor:	without water-in-oil sensor
Temp. sensor:	N
Others:	
Net weight:	142 kg

Description	Value
Controls:	
Control box:	not included
Moisture sensor:	without moisture sensors
Water-in-oil sensor:	without water-in-oil sensor
Temp. sensor:	N
Others:	
Net weight:	142 kg

Description	Value
General information:	
Product name:	SEV.80.80.92.2.51D.R
Product No.:	96889331
EAN:	5700312864479
Price:	On request
Technical:	
Actual calculated flow:	13.8 l/s
Max flow:	25 l/s
Resulting head of the pump:	25.58 m
Head max:	37 m
Type of impeller:	Super Vortex
Maximum particle size:	80 mm
Primary shaft seal:	SIC/SIC
Secondary shaft seal:	CARBON/CERAMICS
Max. hydraulic efficiency:	39 %
Approvals on nameplate:	EN12050-1
Curve tolerance:	ISO9906:2012 3B2
Cooling jacket:	with cooling jacket
Materials:	
Pump housing:	EN 1.4408
Impeller:	Stainless steel
Installation:	
Maximum ambient temperature:	313 K
Maximum operating pressure:	600 kPa
Flange standard:	DIN
Pump outlet:	DN 80
Pressure stage:	PN 10
Maximum installation depth:	20 m
Inst dry/wet:	DRY/SUBMERGED
Installation:	horizontal or vertical
Liquid:	
Pumped liquid:	any viscous fluid
Liquid temperature range:	273 .. 313 K
Density:	998.2 kg/m³
Electrical data:	
Power input - P1:	10.5 kW
Rated power - P2:	9.2 kW
Main frequency:	50 Hz
Rated voltage:	3 x 380-415 V
Voltage tolerance:	+6/-10 %
Max starts per. hour:	20
Rated current:	18,8-17,5 A
Rated current at 3/4 load:	13.4 A
Rated current at 1/2 load:	10 A
Starting current:	162 A
Rated current at no load:	5.6 A
Cos phi - power factor:	0,89
Cos phi - p.f. at no load:	0,16
Cos phi - p.f. at 3/4 load:	0,85
Cos phi - p.f. at 1/2 load:	0,78
Rated speed:	2935 rpm
Locked-rotor torque:	69 Nm
Breakdown torque:	99 Nm
Moment of inertia:	0.0334 kg m²
Motor efficiency at full load:	87.6 %
Motor efficiency at 3/4 load:	87.4 %
Motor efficiency at 1/2 load:	85.4 %
Number of poles:	2
Start. method:	star/delta
Enclosure class (IEC 34-5):	IP68
Insulation class (IEC 85):	F
Explosion proof:	no
Motor protection:	THERMAL SWITCH
Thermal protec:	internal
Length of cable:	10 m
Cable type:	LYNIFLEX
Type of cable plug:	NO PLUG



Description	Value
Controls:	
Control box:	not included
Moisture sensor:	without moisture sensors
Water-in-oil sensor:	without water-in-oil sensor
Temp. sensor:	N
Others:	
Net weight:	190 kg

12 ANNEX III – GRAVITY PROFILE TABLES

K.HIMMAFUSHI			Meters	KM
PS 1	Total Lengths of Catchment (Sewer Main)		4586.09	4.59
Inlet 1				

Profile 1

[illegible]

Profile 2

[illegible]

Profile 3

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-3	MH1-3	41.00	0.004	160	0.831	0.966	0.071	-0.093	0.760	1.059	0.004
TOTAL		41.00									

Profile 4

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-4	MH1-8	44.00	0.004	160	0.693	0.837	-0.067	-0.243	0.760	1.080	0.004
MH1-8	MH1-9	29.00	0.004	160	0.837	0.765	-0.243	-0.359	1.080	1.124	0.004
MH1-9	MH1-5	61.00	0.004	160	0.765	1.026	-0.359	-0.603	1.124	1.629	0.004
TOTAL		134.00									

Profile 5

[illegible]

Profile 6

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-6	MH1-9	56.00	0.004	160	0.701	0.765	-0.059	-0.283	0.760	1.048	0.004
TOTAL		56.00									

Profile 7

[illegible]

Profile 8

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-8	MH1-10	27.00	0.004	160	0.642	0.756	-0.118	-0.226	0.760	0.982	0.004
MH1-10	MH1-11	37.00	0.004	160	0.756	0.813	-0.226	-0.374	0.982	1.187	0.004
MH1-11	MH1-12	29.29	0.004	160	0.813	0.711	-0.374	-0.491	1.187	1.202	0.004
MH1-12	MH1-13	22.62	0.004	160	0.711	0.816	-0.491	-0.582	1.202	1.398	0.004
MH1-13	MH1-14	19.00	0.004	160	0.816	0.722	-0.582	-0.658	1.398	1.380	0.004
MH1-14	MH1-15	19.00	0.004	160	0.722	0.762	-0.658	-0.734	1.380	1.496	0.004
MH1-15	MH1-6	46.00	0.004	160	0.762	1.238	-0.734	-0.918	1.496	2.156	0.004
TOTAL		199.91									

Profile 9

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-9	MH1-16	33.00	0.004	160	0.716	0.700	-0.044	-0.176	0.760	0.876	0.004
MH1-16	MH1-11	30.09	0.004	160	0.700	0.813	-0.176	-0.296	0.876	1.109	0.004
TOTAL		63.09									

Profile 10

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-10	MH1-17	26.00	0.004	160	0.741	0.727	-0.019	-0.123	0.760	0.850	0.004
MH1-17	MH1-12	37.00	0.004	160	0.727	0.711	-0.123	-0.271	0.850	0.982	0.004
TOTAL		63.00									

Profile 11

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-11	MH1-18	26.00	0.004	160	0.700	0.704	-0.060	-0.164	0.760	0.868	0.004
MH1-18	MH1-19	37.00	0.004	160	0.704	0.684	-0.164	-0.312	0.868	0.996	0.004
MH1-19	MH1-13	8.63	0.004	160	0.684	0.816	-0.312	-0.347	0.996	1.163	0.004
TOTAL		71.63									

Profile 12

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-12	MH1-18	53.00	0.004	160	0.947	0.704	0.187	-0.056	0.760	0.760	0.005
TOTAL		53.00									

Profile 13

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-13	MH1-19	15.00	0.004	160	0.758	0.684	-0.002	-0.076	0.760	0.760	0.005
TOTAL		15.00									

Profile 14

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-14	MH1-20	50.00	0.004	160	0.786	0.764	0.026	-0.174	0.760	0.938	0.004
MH1-20	MH1-14	23.50	0.004	160	0.764	0.722	-0.174	-0.268	0.938	0.990	0.004
TOTAL		73.50									

Profile 15

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-15	MH1-21	34.00	0.004	160	0.700	0.658	-0.060	-0.196	0.760	0.854	0.004
MH1-21	MH1-22	43.00	0.004	160	0.658	0.774	-0.196	-0.368	0.854	1.142	0.004
MH1-22	MH1-15	43.00	0.004	160	0.774	0.762	-0.368	-0.540	1.142	1.302	0.004
TOTAL		120.00									

Profile 16

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-16	MH1-23	32.00	0.004	160	1.057	1.146	0.297	0.169	0.760	0.977	0.004
MH1-23	MH1-7	44.00	0.004	160	1.146	1.317	0.169	-0.007	0.977	1.324	0.004
TOTAL		76.00									

Inlet 2

Profile 17

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-17	MH1-24	36.00	0.004	160	0.768	0.943	0.008	-0.136	0.760	1.079	0.004
MH1-24	MH1-25	42.00	0.004	160	0.943	0.639	-0.136	-0.304	1.079	0.943	0.004
MH1-25	MH1-26	46.00	0.004	160	0.639	0.755	-0.304	-0.488	0.943	1.243	0.004
MH1-26	MH1-27	14.00	0.004	160	0.755	0.773	-0.488	-0.544	1.243	1.317	0.004
MH1-27	MH1-28	32.00	0.004	160	0.773	0.925	-0.544	-0.672	1.317	1.597	0.004
MH1-28	MH1-29	33.00	0.004	160	0.925	1.024	-0.672	-0.804	1.597	1.828	0.004
MH1-29	MH1-30	11.00	0.004	160	1.024	1.072	-0.804	-0.848	1.828	1.920	0.004
MH1-30	MH1-31	10.50	0.004	160	1.072	1.134	-0.848	-0.890	1.920	2.024	0.004
MH1-31	MH1-32	21.00	0.004	160	1.134	1.132	-0.890	-0.974	2.024	2.106	0.004
MH1-32	MH1-33	34.17	0.004	160	1.132	1.436	-0.974	-1.111	2.106	2.547	0.004
MH1-33	PS1-2	50.16	0.004	160	1.436	1.332	-1.111	-1.311	2.547	2.643	0.004
TOTAL		329.83									

Profile 18

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-18	MH1-34	29.00	0.004	160	0.835	0.750	0.075	-0.041	0.760	0.791	0.004
MH1-34	MH1-25	39.00	0.004	160	0.750	0.639	-0.041	-0.197	0.791	0.836	0.004
TOTAL		68.00									

Profile 19

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-19	MH1-34	29.00	0.004	160	0.837	0.750	0.077	-0.039	0.760	0.789	0.004
TOTAL		29.00									

Profile 20

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-20	MH1-35	30.00	0.004	160	0.750	0.635	-0.010	-0.130	0.760	0.765	0.004
MH1-35	MH1-26	37.00	0.004	160	0.635	0.755	-0.130	-0.278	0.765	1.033	0.004
TOTAL		67.00									

Profile 21

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	

CO1-21	MH1-36	16.50	0.004	160	0.789	0.642	0.029	-0.118	0.760	0.760	0.009
MH1-36	MH1-37	24.16	0.004	160	0.642	0.606	-0.118	-0.215	0.760	0.821	0.004
MH1-37	MH1-27	50.80	0.004	160	0.606	0.773	-0.215	-0.418	0.821	1.191	0.004
TOTAL		91.46									

Profile 22

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-22	MH1-37	15.50	0.004	160	0.639	0.606	-0.121	-0.183	0.760	0.789	0.004
TOTAL		15.50									

Profile 23

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-23	MH1-38	26.00	0.004	160	0.762	0.669	0.002	-0.102	0.760	0.771	0.004
MH1-38	MH1-39	15.00	0.004	160	0.669	0.543	-0.102	-0.217	0.771	0.760	0.008
MH1-39	MH1-40	23.50	0.004	160	0.543	0.621	-0.217	-0.311	0.760	0.932	0.004
MH1-40	MH1-41	53.38	0.004	160	0.621	0.862	-0.311	-0.525	0.932	1.387	0.004
MH1-41	MH1-29	35.00	0.004	160	0.862	1.024	-0.525	-0.665	1.387	1.689	0.004
TOTAL		152.88									

Profile 24

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-24	MH1-39	13.00	0.004	160	0.642	0.543	-0.118	-0.217	0.760	0.760	0.008
TOTAL		13.00									

Profile 25

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-25	MH1-40	30.00	0.004	160	0.701	0.621	-0.059	-0.179	0.760	0.800	0.004
TOTAL		30.00									

Profile 26

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-26	MH1-40	17.00	0.004	160	0.667	0.621	-0.093	-0.161	0.760	0.782	0.004
TOTAL		17.00									

Profile 27

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-27	MH1-41	25.00	0.004	160	0.773	0.862	0.013	-0.087	0.760	0.949	0.004
TOTAL		25.00									

Profile 28

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-28	MH1-41	25.00	0.004	160	0.939	0.862	0.179	0.079	0.760	0.783	0.004
TOTAL		25.00									

Profile 29

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
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From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-29	MH1-42	38.00	0.004	160	0.652	0.763	-0.108	-0.260	0.760	1.023	0.004
MH1-42	MH1-43	51.00	0.004	160	0.763	1.064	-0.260	-0.464	1.023	1.528	0.004
MH1-43	MH1-44	30.00	0.004	160	1.064	1.140	-0.464	-0.584	1.528	1.724	0.004
MH1-44	MH1-45	38.26	0.004	160	1.140	1.306	-0.584	-0.737	1.724	2.043	0.004
MH1-45	MH1-46	17.00	0.004	160	1.306	1.337	-0.737	-0.805	2.043	2.142	0.004
MH1-46	MH1-47	17.00	0.004	160	1.337	1.406	-0.805	-0.873	2.142	2.279	0.004
MH1-47	MH1-33	11.67	0.004	160	1.406	1.436	-0.873	-0.920	2.279	2.356	0.004
TOTAL		202.93									

Profile 30

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-30	MH1-48	37.50	0.004	160	0.694	0.815	-0.066	-0.216	0.760	1.031	0.004
MH1-48	MH1-49	25.00	0.004	160	0.815	0.830	-0.216	-0.316	1.031	1.146	0.004
MH1-49	MH1-44	53.00	0.004	160	0.830	1.140	-0.316	-0.528	1.146	1.668	0.004
TOTAL		115.50									

Profile 31

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-31	MH1-48	29.00	0.004	160	0.897	0.815	0.137	0.021	0.760	0.794	0.004
TOTAL		29.00									

Profile 32

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-32	MH1-49	34.00	0.004	160	0.774	0.830	0.014	-0.122	0.760	0.952	0.004
TOTAL		34.00									

Profile 33

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-33	MH1-49	29.00	0.004	160	0.876	0.830	0.116	0.000	0.760	0.830	0.004
TOTAL		29.00									

Profile 34

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-34	MH1-50	30.00	0.004	160	0.956	0.913	0.196	0.076	0.760	0.837	0.004
MH1-50	MH1-51	54.00	0.004	160	0.913	1.119	0.076	-0.140	0.837	1.259	0.004
MH1-51	MH1-44	34.70	0.004	160	1.119	1.140	-0.140	-0.279	1.259	1.419	0.004
TOTAL		118.70									

Profile 35

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-35	MH1-52	30.00	0.004	160	1.123	1.285	0.363	0.243	0.760	1.042	0.004
MH1-52	MH1-50	23.00	0.004	160	1.285	0.913	0.243	0.151	1.042	0.762	0.004
TOTAL		53.00									

Profile 36

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	

CO1-36	MH1-51	22.00	0.004	160	0.968	1.119	0.208	0.120	0.760	0.999	0.004
TOTAL		22.00									

Profile 37

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-37	MH1-45	58.00	0.004	160	1.002	1.306	0.242	0.010	0.760	1.296	0.004
TOTAL		58.00									

Profile 38

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-35	MH1-53	20.50	0.004	160	1.241	1.246	0.481	0.399	0.760	0.847	0.004
MH1-53	MH1-46	39.00	0.004	160	1.246	1.337	0.399	0.243	0.847	1.094	0.004
TOTAL		59.50									

Profile 39

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-39	MH1-47	30.50	0.004	160	1.366	1.406	0.606	0.484	0.760	0.922	0.004
TOTAL		30.50									

Inlet 3

Profile 40

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-40	MH1-54	18.00	0.004	160	1.033	1.009	0.273	0.201	0.760	0.808	0.004
MH1-54	MH1-55	33.00	0.004	160	1.009	0.996	0.201	0.069	0.808	0.927	0.004
MH1-55	MH1-56	52.00	0.004	160	0.996	0.911	0.069	-0.139	0.927	1.050	0.004
MH1-56	MH1-57	9.00	0.004	160	0.911	0.931	-0.139	-0.175	1.050	1.106	0.004
MH1-57	MH1-58	37.00	0.004	160	0.931	1.183	-0.175	-0.323	1.106	1.506	0.004
MH1-58	MH1-59	35.00	0.004	160	1.183	1.340	-0.323	-0.463	1.506	1.803	0.004
MH1-59	MH1-60	60.00	0.004	160	1.340	1.203	-0.463	-0.703	1.803	1.906	0.004
MH1-60	MH1-61	37.40	0.004	161	1.203	1.074	-0.703	-0.853	1.906	1.927	0.004
MH1-61	MH1-62	32.00	0.004	162	1.074	1.182	-0.853	-0.981	1.927	2.163	0.004
MH1-62	MH1-63	10.00	0.004	163	1.182	1.373	-0.981	-1.021	2.163	2.394	0.004
MH1-63	PS1-3	19.76	0.004	164	1.373	1.332	-1.021	-1.100	2.394	2.432	0.004
TOTAL		343.16									

Profile 41

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-41	MH1-64	27.00	0.004	160	1.046	1.123	0.286	0.178	0.760	0.945	0.004
MH1-64	MH1-55	22.00	0.004	160	1.123	0.996	0.178	0.090	0.945	0.906	0.004
TOTAL		49.00									

Profile 42

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO1-42	MH1-65	46.00	0.004	160	0.987	0.868	0.227	0.043	0.760	0.825	0.004
MH1-65	MH1-56	40.00	0.004	160	0.868	0.911	0.043	-0.117	0.825	1.028	0.004
TOTAL		86.00									

Profile 43

[illegible][illegible][illegible][illegible][illegible][illegible]

K.HIMMAFUSHI			Meters	KM
PS 2	Total Lengths of Catchment (Sewer Main)		3404.00	3.40

Inlet 1

Profile 1

[illegible]

Profile 2

[illegible]

Profile 3

[illegible]

Profile 4

[illegible]

Profile 5

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-5	MH2-3	50.00	0.004	160	0.543	0.582	-0.217	-0.417	0.760	0.999	0.004
TOTAL		50.00									

Profile 6

[illegible]

Profile 7

[illegible]

Profile 8

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-8	MH2-5	31.00	0.004	160	0.514	0.554	-0.246	-0.370	0.760	0.924	0.004
TOTAL		31.00									

Profile 9

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-9	MH2-5	34.00	0.004	160	0.914	0.554	0.154	-0.206	0.760	0.760	0.011
TOTAL		34.00									

Profile 10

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-10	MH2-8	34.000	0.004	160	0.528	0.511	-0.232	-0.368	0.760	0.879	0.004
MH2-8	MH2-9	17.000	0.004	160	0.511	0.511	-0.368	-0.436	0.879	0.947	0.004
MH2-9	MH2-10	37.000	0.004	160	0.511	0.767	-0.436	-0.584	0.947	1.351	0.004
MH2-10	MH2-11	28.000	0.004	160	0.767	1.056	-0.584	-0.696	1.351	1.752	0.004
MH2-11	MH2-12	30.000	0.004	160	1.056	0.870	-0.696	-0.816	1.752	1.686	0.004
MH2-12	MH2-13	30.000	0.004	160	0.870	0.580	-0.816	-0.936	1.686	1.516	0.004
MH2-13	MH2-6	39.000	0.004	160	0.580	0.762	-0.936	-1.092	1.516	1.854	0.004
TOTAL		215.00									

Profile 11

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-11	MH2-9	34.00	0.004	160	0.655	0.511	-0.105	-0.249	0.760	0.760	0.004
TOTAL		34.00									

Profile 12

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-5	MH2-10	34.00	0.004	160	0.961	0.767	0.201	0.007	0.760	0.760	0.006
TOTAL		34.00									

Profile 13

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-13	MH2-11	34.00	0.004	160	0.884	1.056	0.124	-0.012	0.760	1.068	0.004
TOTAL		34.00									

Profile 14

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-14	MH2-12	34.00	0.004	160	0.766	0.870	0.006	-0.130	0.760	1.000	0.004
TOTAL		34.00									

Profile 15

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
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From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-15	MH2-14	20.00	0.004	160	0.447	0.597	-0.313	-0.393	0.760	0.990	0.004
MH2-14	MH2-15	33.00	0.004	160	0.597	0.696	-0.393	-0.525	0.990	1.221	0.004
MH2-15	MH2-13	32.00	0.004	160	0.696	0.580	-0.525	-0.653	1.221	1.233	0.004
TOTAL		85.00									

Profile 16

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-16	MH2-16	37.00	0.004	160	0.665	0.674	-0.095	-0.243	0.760	0.917	0.004
MH2-16	MH2-14	20.00	0.004	160	0.674	0.597	-0.243	-0.323	0.917	0.920	0.004
TOTAL		57.00									

Profile 17

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-17	MH2-16	22.00	0.004	160	0.712	0.674	-0.048	-0.136	0.760	0.810	0.004
TOTAL		22.00									

Profile 18

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-18	MH2-14	34.00	0.004	160	0.678	0.597	-0.082	-0.218	0.760	0.815	0.004
TOTAL		34.00									

Profile 19

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-19	MH2-15	28.00	0.004	160	0.594	0.696	-0.166	-0.278	0.760	0.974	0.004
TOTAL		28.00									

Profile 20

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-20	MH2-15	34.00	0.004	160	0.776	0.696	0.016	-0.120	0.760	0.816	0.004
TOTAL		34.00									

Profile 21

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-21	MH2-13	34.00	0.004	160	0.689	0.580	-0.071	-0.207	0.760	0.787	0.004
TOTAL		34.00									

Profile 22

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-22	MH2-6	27.00	0.004	160	0.748	0.762	-0.012	-0.120	0.760	0.882	0.004
TOTAL		27.00									

Inlet 2

Profile 23

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-23	MH2-17	55.00	0.004	160	0.790	0.804	0.030	-0.190	0.760	0.994	0.004
MH2-17	MH2-18	55.00	0.004	160	0.804	0.681	-0.190	-0.410	0.994	1.091	0.004
MH2-18	MH2-19	48.00	0.004	160	0.681	0.747	-0.410	-0.602	1.091	1.349	0.004
MH2-19	MH2-20	60.00	0.004	160	0.747	0.889	-0.602	-0.842	1.349	1.731	0.004
MH2-20	MH2-21	60.00	0.004	160	0.889	0.895	-0.842	-1.082	1.731	1.977	0.004
MH2-21	MH2-22	40.00	0.004	160	0.895	0.531	-1.082	-1.242	1.977	1.773	0.004
MH2-22	MH2-23	31.00	0.004	160	0.531	0.708	-1.242	-1.366	1.773	2.074	0.004
MH2-23	PS2-2	31.00	0.004	161	0.708	0.716	-1.366	-1.490	2.074	2.206	0.004
TOTAL		380.00									

Profile 24

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-24	MH2-24	25.00	0.004	160	0.807	0.726	0.047	-0.053	0.760	0.779	0.004
MH2-24	MH2-25	44.00	0.004	160	0.726	0.742	-0.053	-0.229	0.779	0.971	0.004
MH2-25	MH2-26	44.00	0.004	160	0.742	0.850	-0.229	-0.405	0.971	1.255	0.004
MH2-26	MH2-19	25.00	0.004	160	0.850	0.747	-0.405	-0.505	1.255	1.252	0.004
TOTAL		138.00									

Profile 25

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-25	MH2-27	40.00	0.004	160	0.972	1.090	0.212	0.052	0.760	1.038	0.004
MH2-27	MH2-28	40.00	0.004	160	1.090	1.045	0.052	-0.108	1.038	1.153	0.004
MH2-28	MH2-29	35.00	0.004	160	1.045	1.066	-0.108	-0.248	1.153	1.314	0.004
MH2-29	MH2-26	35.00	0.004	160	1.066	0.850	-0.248	-0.388	1.314	1.238	0.004
TOTAL		150.00									

Profile 26

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-26	MH2-30	55.00	0.004	160	1.100	1.178	0.340	0.120	0.760	1.058	0.004
MH2-30	MH2-31	55.00	0.004	160	1.178	0.988	0.120	-0.100	1.058	1.088	0.004
MH2-31	MH2-32	48.00	0.004	160	0.988	0.721	-0.100	-0.292	1.088	1.013	0.004
MH2-32	MH2-21	48.00	0.004	160	0.721	0.895	-0.292	-0.484	1.013	1.379	0.004
TOTAL		206.00									

Profile 27

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-27	MH2-21	57.00	0.004	160	0.896	0.895	0.136	-0.092	0.760	0.987	0.004
TOTAL		57.00									

Profile 28

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO2-28	MH2-33	60.00	0.004	160	0.676	0.838	-0.084	-0.324	0.760	1.162	0.004
MH2-33	MH2-34	35.00	0.004	160	0.838	0.972	-0.324	-0.464	1.162	1.436	0.004
MH2-34	MH2-35	35.00	0.004	160	0.972	0.907	-0.464	-0.604	1.436	1.511	0.004
MH2-35	MH2-36	50.00	0.004	160	0.907	0.543	-0.604	-0.804	1.511	1.347	0.004
MH2-36	MH2-22	42.00	0.004	160	0.543	0.531	-0.804	-0.972	1.347	1.503	0.004

K.HIMMAFUSHI

Meters

KM

PS 3

Total Lengths of Catchment (Sewer Main)

2484.50

2.48

Inlet 1

Profile 1

[illegible]

Inlet 2

Profile 2

[illegible]

Inlet 3

Profile 3

[illegible]

Inlet 4

Profile 4

[illegible]

Profile 5

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO3-5	MH3-24	60.00	0.004	160	1.600	1.600	0.840	0.600	0.760	1.000	0.004
MH3-24	MH3-25	60.00	0.004	160	1.600	1.600	0.600	0.360	1.000	1.240	0.004
MH3-25	MH3-26	60.00	0.004	160	1.600	1.600	0.360	0.120	1.240	1.480	0.004
MH3-26	MH3-27	55.00	0.004	160	1.600	1.600	0.120	-0.100	1.480	1.700	0.004
MH3-27	MH3-20	55.00	0.004	160	1.600	1.600	-0.100	-0.320	1.700	1.920	0.004
TOTAL		290.00									

Profile 6

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO3-6	MH3-28	47.00	0.004	160	1.600	1.600	0.840	0.652	0.760	0.948	0.004
MH3-28	MH3-29	47.00	0.004	160	1.600	1.600	0.652	0.464	0.948	1.136	0.004
MH3-29	MH3-30	60.00	0.004	160	1.600	1.600	0.464	0.224	1.136	1.376	0.004
MH3-30	MH3-31	55.00	0.004	160	1.600	1.600	0.224	0.004	1.376	1.596	0.004
MH3-31	MH3-32	54.00	0.004	160	1.600	1.600	0.004	-0.212	1.596	1.812	0.004
MH3-32	MH3-33	23.00	0.004	160	1.600	1.600	-0.212	-0.304	1.812	1.904	0.004
MH3-33	MH3-34	35.00	0.004	160	1.600	1.600	-0.304	-0.444	1.904	2.044	0.004
MH3-34	MH3-22	39.00	0.004	161	1.600	1.600	-0.444	-0.600	2.044	2.200	0.004
TOTAL		360.00									

Profile 7

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO3-7	MH3-29	60.00	0.004	160	1.600	1.600	0.840	0.600	0.760	1.000	0.004
TOTAL		60.00									

Profile 8

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO3-8	MH3-35	36.00	0.004	160	1.600	1.600	0.840	0.696	0.760	0.904	0.004
MH3-35	MH3-32	36.00	0.004	160	1.600	1.600	0.696	0.552	0.904	1.048	0.004
TOTAL		72.00									

Profile 9

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO3-9	MH3-36	35.00	0.004	160	1.600	1.600	0.840	0.700	0.760	0.900	0.004
MH3-36	MH3-37	35.00	0.004	160	1.600	1.600	0.700	0.560	0.900	1.040	0.004
MH3-37	MH3-38	39.00	0.004	160	1.600	1.600	0.560	0.404	1.040	1.196	0.004
MH3-38	MH3-33	38.50	0.004	160	1.600	1.600	0.404	0.250	1.196	1.350	0.004
TOTAL		147.50									

Profile 10

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO3-10	MH3-37	51.00	0.004	160	1.600	1.600	0.840	0.636	0.760	0.964	0.004
TOTAL		51.00									

Profile 11

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
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[illegible]

K.HIMMAFUSHI							Meters		KM	
PS 4				Total Lengths of Catchment (Sewer Main)			3883.00		3.88	

Inlet 1

Profile 1

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO4-1	MH4-1	35.000	0.004	160	1.600	1.600	0.840	0.700	0.760	0.900	0.004
MH4-1	MH4-2	35.000	0.004	160	1.600	1.600	0.700	0.560	0.900	1.040	0.004
MH4-2	MH4-3	59.000	0.004	160	1.600	1.600	0.560	0.324	1.040	1.276	0.004
MH4-3	MH4-4	59.000	0.004	160	1.600	1.600	0.324	0.088	1.276	1.512	0.004
MH4-4	MH4-5	59.000	0.004	160	1.600	1.600	0.088	-0.148	1.512	1.748	0.004
MH4-5	MH4-6	38.000	0.004	160	1.600	1.600	-0.148	-0.300	1.748	1.900	0.004
MH4-6	MH4-7	38.000	0.004	160	1.600	1.600	-0.300	-0.452	1.900	2.052	0.004
MH4-7	PS4-1	37.000	0.004	160	1.600	1.600	-0.452	-0.600	2.052	2.200	0.004
TOTAL		360.00									

Profile 2

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO4-2	MH4-8	60.00	0.004	160	1.600	1.600	0.840	0.600	0.760	1.000	0.004
MH4-8	MH4-9	53.00	0.004	160	1.600	1.600	0.600	0.388	1.000	1.212	0.004
MH4-9	MH4-10	53.00	0.004	160	1.600	1.600	0.388	0.176	1.212	1.424	0.004
MH4-10	MH4-11	35.00	0.004	160	1.600	1.600	0.176	0.036	1.424	1.564	0.004
MH4-11	MH4-5	35.00	0.004	160	1.600	1.600	0.036	-0.104	1.564	1.704	0.004
TOTAL		236.00									

Profile 3

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO4-3	MH4-12	50.00	0.004	160	1.600	1.600	0.840	0.640	0.760	0.960	0.004
MH4-12	MH4-13	50.00	0.004	160	1.600	1.600	0.640	0.440	0.960	1.160	0.004
MH4-13	MH4-10	50.00	0.004	160	1.600	1.600	0.440	0.240	1.160	1.360	0.004
TOTAL		150.00									

Profile 4

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO4-4	MH4-14	38.00	0.004	160	1.600	1.600	0.840	0.688	0.760	0.912	0.004
MH4-14	MH4-15	38.00	0.004	160	1.600	1.600	0.688	0.536	0.912	1.064	0.004
MH4-15	MH4-5	38.00	0.004	160	1.600	1.600	0.536	0.384	1.064	1.216	0.004
TOTAL		114.00									

Inlet 2

Profile 5

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO4-5	MH4-16	60.00	0.004	160	1.600	1.600	0.840	0.600	0.760	1.000	0.004
MH4-16	MH4-17	60.00	0.004	160	1.600	1.600	0.600	0.360	1.000	1.240	0.004
MH4-17	MH4-18	55.00	0.004	160	1.600	1.600	0.360	0.140	1.240	1.460	0.004
MH4-18	MH4-19	27.00	0.004	160	1.600	1.600	0.140	0.032	1.460	1.568	0.004
MH4-19	MH4-20	49.00	0.004	160	1.600	1.600	0.032	-0.164	1.568	1.764	0.004
MH4-20	MH4-21	40.00	0.004	160	1.600	1.600	-0.164	-0.324	1.764	1.924	0.004
MH4-21	MH4-22	34.00	0.004	160	1.600	1.600	-0.324	-0.460	1.924	2.060	0.004
MH4-22	MH4-23	34.00	0.004	160	1.600	1.600	-0.460	-0.596	2.060	2.196	0.004

[illegible]

Profile 14

[illegible]

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
C04-15	MH4-34	47.00	0.004	160	1.600	1.600	0.840	0.652	0.760	0.948	0.004
TOTAL		47.00									

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
C04-16	MH4-35	32.00	0.004	160	1.600	1.600	0.840	0.712	0.760	0.888	0.004
TOTAL		32.00									

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Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO4-19	MH4-46	58.00	0.004	160	1.600	1.600	0.840	0.608	0.760	0.992	0.004
TOTAL		58.00									

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO4-20	MH4-41	37.00	0.004	160	1.600	1.600	0.840	0.692	0.760	0.908	0.004
TOTAL		37.00									

Profile 21

[illegible][illegible][illegible][illegible]

Profile 25

Manholes		Pipe Length	Design Slope	Size	Ground Level (MSL)		Invert Level (MSL)		Pipe Depth (m)		Slope
From	To	m	1 : 250	mm dia (OD)	UG	LG	UI	LI	UI	LI	
CO4-25	MH4-62	51.00	0.004	160	1.600	1.600	0.840	0.636	0.760	0.964	0.004
TOTAL		51.00									

13 ANNEX IV – CONCEPT APPROVAL LETTER



Ministry of Environment and Energy
Male', Republic of Maldives.

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Date: 26 June 2016

No: 438-PDU/PRIV/2016/657

Mr. Ibrahim Hameez,
Managing Director,
Riyan Pvt Ltd,
Male' Maldives.

Dear Mr. Ibrahim,

Project: Sanitation Facilities in 05 (Five) Islands Project.

Subject: Concept Design of K.Himmafushi, K.Thulusdhoo and Hdh.Nolhivaranfaru (Lot-2).

Reference is made to the Concept Design of K.Himmafushi, K.Thulusdhoo and Hdh.Nolhivaranfaru dated 15th May 2016 and submitted on 30th May 2016.

We hereby approve the Concept Design of Gravity Type Wastewater Collection, Treatment and Disposal System in K.Himmafushi, K.Thulusdhoo and Hdh.Nolhivaranfaru and request you to complete the Environmental Impact Assessment (EIA) report in accordance to Environmental Protection Agency's (EPA) Guidelines and the bidding documents.

Please note that as per the Consultant TOR, Reporting Requirement and Time Schedule for Deliverables, the consultant is required to submit the Concept Design in **3 hard copies and soft copy in English** as the final report.

Sincerely,

Shaheedha Adam Ibrahim,
Director General.



Green Building, Handhuvaaree Hingun,
Maafannu, Male', 20392, Republic of Maldives.

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www.facebook.com/environment.gov.mv



Ministry of Environment and Energy

Republic of Maldives

Concept Design Approval Form

Consultant Name:	Riyan Pvt Ltd	Name of the Island:	K.Himmafushi
Project Name:	Sanitation Facilities in Five Islands Project	Submission Date:	30/05/2016

Remarks:

- Himmafushi reclaiming work has not yet started. Need to coordinate with Utility companies and council regarding power distribution and access to site after the reclamation.
- For sustainability of the facility, the design need to see possibilities of solar integration. Hence the electrical drawings need to cater the requirement.
- For safety the panel board and generator unit need to be compartmentalized and separated.
- The boundary of the facility need to be secure and long lasting.

Reviewed by:

Name:	Signature:	Designation/Department:	Date:
Afsal Hussain		Asst. Director	26-06-2016
Hannan Ahmed		OFID Coordinator	26-06-2016

Approved by:

Name:	Signature:	Designation/Department:	Date:
MUAZ MOHAMED		Asst. Project Coordinator	26/06/2016